

**RIGID FRAME:**

**BASIC COLUMN REACTIONS (k )**

Frame Line	Column Line	Dead	Collateral	Live	Snow	Wind_L1	Wind_R1
1 * B		-0.01	2.64	0.21	1.17	-0.23	10.05
1 * A		0.01	1.59	-0.21	1.13	0.23	6.15
2 B		0.03	2.92	0.25	1.44	-0.07	11.28
2 A		-0.03	1.84	-0.25	1.40	0.07	7.27

Frame Line	Column Line	Wind_L2	Wind_R2	LnWind_1	LnWind_2	Seismic_L	Seismic_R
1 * B		-4.06	-21.23	3.78	-14.80	0.62	-7.53
1 * A		-4.69	-4.76	2.37	-11.20	-0.82	-7.30
2 B		-4.23	-20.99	4.01	-14.28	1.14	-7.30
2 A		-4.77	-4.13	2.58	-10.83	-1.26	-7.18

Frame Line	Column Line	LnSeis
1 * B		0.00
1 * A		0.00
2 B		0.00
2 A		0.00

1 \* Frame lines: 1 3

**RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES**

Frm Line	Col Line	Load Id	Hmax	Vmax	Load Id	Hmin	Vmin	Anc. No	Bolt D(in)	Base Plate Wid	Len	Thk	Grout (in)
1 * B		8	4.6	-12.7	2	-4.2	8.2	4	0.750	8.000	13.00	0.500	0.0
1 * A		3	3.0	5.0	7	-5.5	-3.8	4	0.750	8.000	13.00	0.500	0.0

1 \* Frame lines: 1 3

**RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES**

Frm Line	Col Line	Load Id	Hmax	Vmax	Load Id	Hmin	Vmin	Anc. No	Bolt D(in)	Base Plate Wid	Len	Thk	Grout (in)
2 B		8	5.2	-12.1	6	-4.2	-19.2	4	0.750	8.000	13.00	0.500	0.0
2 A		3	2.8	7.2	7	-5.9	-3.1	4	0.750	8.000	13.00	0.500	0.0

**WIND BENT REACTIONS**

Wall Loc	Line	Col Line	Wind Horz	Wind Vert	Seismic Horz	Seismic Vert
F_SW	A	2	1.35	1.85	0.69	0.95
F_SW	A	3	1.35	1.85	0.69	0.95
B_SW	B	3	1.22	1.79	0.86	1.27
B_SW	B	2	1.22	1.79	0.86	1.27

**BRACING REACTIONS, PANEL SHEAR**

Wall Loc	Line	Col Line	Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel Shear (lb/ft)
L_EW	1						Rigid Frame At Endwall
F_SW	A						Wind Bent In Wall
R_EW	3						Rigid Frame At Endwall
B_SW	B						Wind Bent In Wall

**NOTES FOR REACTIONS**

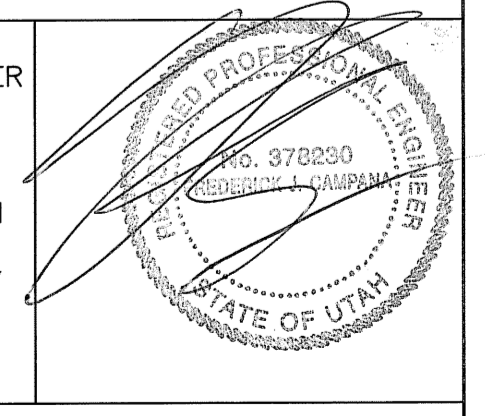
- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data.
 

Width (ft)	: 26
Length (ft)	: 50
Eave Height (ft)	: 18.083 / 17
Roof Slope (rise/12)	: 0.5:12
Design Code	: IBC 06
Enclosure	: Closed
Dead Load (psf)	: 2.5
Collateral Load (psf)	: 3
Wind Speed (mph)	: 90 mph
Wind Importance Factor	: 1.00
Wind Exposure	: C
Live Load (psf)	: 20
Frame Live Load (psf)	: 12
Ground Snow Load (psf)	: 21.000
Roof Snow Load (psf)	: 20
Snow Exposure	: 1.000
Snow Importance Factor	: 1.000
Thermal Factor	: 1.000
Seismic Importance Factor	: 1.00
Spectral Response Accel.	: Ss=0.511 ; S1=0.156
Spectral Response Coeff.	: Sds=0.474 ; Sd1=0.226
Seismic Coeff. (Fa*Ss)	: 0.710 ; Fa=1.391
Seismic Design Category	: D
- Loading conditions are:
  - 1 DL+CL+SL
  - 2 DL+CL+0.75SL+0.75WL2
  - 3 DL+CL+0.75SL+0.75WR2
  - 4 0.60DL+WL1
  - 5 0.60DL+WR1
  - 6 0.60DL+WL2
  - 7 DL+CL+WL1
  - 8 DL+CL+WR1

**FOR PERMIT**

JUN 10 2008

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ISSUE	REV.	DESCRIPTION	DATE	DRN. DES.
A		CONFIRMATION	05.20.08	FFF



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DESCRIPTION	FRAME REACTIONS
CUSTOMER	ELEGANTE ENTERPRISES, INC.
END USER	PAINTERS SERVICE CENTER
END USE	OFFICE/SERVICE BAYS
LOCATION	1600 HILTON DR. ST. GEORGE, UT 84770
DRN. BY:	FFF
DATE:	05/20/08
CHK. BY:	
DATE:	
JOB NO.:	28268-D
SCALE:	N.T.S.
DWG. NO.:	F3 OF 3
ISSUE:	A