

**RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES**

Frm Line	Col Line	Load Id	Hmax H	Column Reactions (k)			Anc. No	Bolt D(in)	Base Plate (in)			Grout (in)	
				Vmax	Load Id	Hmin H			Vmin	Wid	Len		Thk
I * 11	2	8	6.9	-8.1	5	-5.2	-15.8	4	0.750	8.000	13.00	0.500	0.0
		2	1.1	30.1	3	-4.4	-18.7						
I * 7	2	6	5.8	-11.8	7	-5.0	-4.0	4	0.750	8.000	13.00	0.500	0.0
		2	-1.1	21.1	4	5.1	-14.7						

I \* Frame lines: I J

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				Vmax	Load Id	Hmin H			Vmin	Wid	Len		Thk
K 11	2	8	5.0	-6.2	5	-3.6	-11.2	4	0.750	8.000	11.00	0.500	0.0
		2	0.7	22.1	3	-3.0	-13.3						
K 7	2	6	3.7	-8.2	7	-3.1	-3.2	4	0.750	8.000	11.00	0.500	0.0
		2	-0.7	15.5	4	3.1	-10.3						

**NOTES FOR REACTIONS**

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data.

Width (ft)	: 30
Length (ft)	: 119
Eave Height (ft)	: 17 / 18.25
Roof Slope (rise/12)	: 0.5:12 /
Design Code	: IBC 06
Enclosure	: Closed
Dead Load (psf)	: 2.4
Collateral Load (psf)	: 3
Wind Speed (mph)	: 90 mph
Wind Importance Factor	: 1.00
Wind Exposure	: C
Live Load (psf)	: 20
Frame Live Load (psf)	: 12
Ground Snow Load (psf)	: 21.000
Roof Snow Load (psf)	: 20
Snow Exposure	: 1.000
Snow Importance Factor	: 1.000
Thermal Factor	: 1.000
Seismic Importance Factor	: 1.00
Spectral Response Accel.	: Ss=0.511 : S1=0.156
Spectral Response Coeff.	: Sds=0.474 : Sd1=0.226
Seismic Coeff. (Fa*Ss)	: 0.710 : Fa=1.391
Seismic Design Category	: D

5. Loading conditions are:

- DL+CL+LL
- DL+CL+SL
- 0.60DL+WL1
- 0.60DL+WR1
- 0.60DL+WL2
- 0.60DL+WR2
- DL+CL+WL1
- DL+CL+WR1
- 0.60DL+WR1+WS
- 0.60DL+WP+LnWnd1
- DL+CL+0.75LL+0.75WL2+0.75WS
- DL+CL+0.75SL+0.75WL2+0.75WS
- DL+CL+0.75LL+0.75WR2+0.75WS
- DL+CL+0.75SL+0.75WR2+0.75WS

**RIGID FRAME:**

**BASIC COLUMN REACTIONS (k)**

Frame Line	Column Line	---Dead---		---Collateral---		---Live---		---Snow---		---Wind_L1---		---Wind_R1---	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
I *	11	0.24	2.48	0.39	1.56	0.92	10.93	0.47	26.06	-4.55	-20.22	6.25	-12.11
I *	7	-0.24	2.40	-0.39	1.57	-0.92	8.28	-0.47	17.14	-4.36	-7.99	5.25	-16.09
K	11	0.16	1.92	0.25	1.13	0.59	7.99	0.27	19.08	-3.09	-14.42	4.55	-9.26
K	7	-0.16	1.86	-0.25	1.14	-0.59	6.04	-0.27	12.54	-2.74	-6.23	3.18	-11.39

Frame Line	Column Line	---Wind_L2---		---Wind_R2---		---LnWind_1---		---LnWind_2---		---Seismic_L---		---Seismic_R---	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
I *	11	-5.32	-17.33	5.55	-9.24	0.83	-7.89	0.83	-7.89	-1.20	-1.21	1.20	1.21
I *	7	-3.57	-5.12	5.95	-13.22	-0.67	-8.11	-0.67	-8.11	-0.99	1.21	0.99	-1.21
K	11	-3.72	-12.32	3.98	-7.17	0.76	-4.98	0.76	-4.98	-0.80	-0.73	0.80	0.73
K	7	-2.10	-4.15	3.76	-9.30	-0.65	-5.12	-0.65	-5.12	-0.52	0.73	0.52	-0.73

Frame Line	Column Line	---LnSeis---	
		Horiz	Vert
I *	11	0.00	-1.61
I *	7	0.00	-1.76
K	11	0.00	0.00
K	7	0.00	0.00

I \* Frame lines: I J

**ENDWALL COLUMN: REACTIONS, ANCHOR BOLTS, & BASE PLATES**

Frm Line	Col Line	Dead Vert	Coll Vert	Live Vert	Column Reactions (k)		Out-Of-Plane			
					Wind-Left Horiz	Wind-Right Horiz	Wd P Horiz	Wd S Horiz		
H	11	1.4	-0.1	7.8	1.8	-25.4	0.0	-10.4	-0.3	0.3 (SOLDIER COLUMN)
H	7	0.3	-0.1	0.5	0.0	-1.1	2.4	-21.0	-0.3	0.4 (SOLDIER COLUMN)
L	7	0.7	0.2	2.3	0.0	-2.4	0.0	-2.4	-0.9	1.0
L	9	0.8	0.6	2.4	1.4	-2.9	0.0	-1.5	-1.5	1.7
L	11	1.1	0.2	4.5	0.0	-5.1	0.9	-6.1	-0.7	0.8

**ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES**

Frm Line	Col Line	Load Id	Hmax H	Column Reactions (k)			Anc. No	Bolt D(in)	Base Plate (in)			Grout (in)	
				Vmax	Load Id	Hmin H			Vmin	Wid	Len		Thk
H 11	2	9	0.3	-9.6	10	-0.3	1.5	4	0.750	8.000	9.000	0.375	0.0
		2	0.0	16.1	11	0.3	-14.8						
H 7	10	9	0.4	-0.9	10	-0.3	0.8	4	0.750	8.000	9.000	0.500	0.0
		2	-0.3	0.8	14	0.3	-15.8						
L 7	2	9	1.0	-2.4	10	-0.9	-0.8	4	0.750	8.000	8.500	0.375	0.0
		2	0.0	4.8	9	1.0	-2.4						
L 9	1	9	1.6	-1.6	10	-1.5	-2.7	4	0.750	8.000	8.500	0.375	0.0
		1	0.0	4.2	10	0.0	-5.1						
L 11	2	9	0.8	-5.1	10	-0.7	-0.5	4	0.750	8.000	8.500	0.375	0.0
		2	-0.1	10.0	9	0.8	-5.1						

**BRACING REACTIONS, PANEL SHEAR**

Loc	Wall Line	Col Line	± Reactions (k)				Panel Shear (lb/ft)
			---Wind---	---Seismic---	Horz	Vert	
L_EW	H		Bracing In Roof To Rigid Frame				
F_SW	7		Wind Bent In Wall				
R_EW	L	9,11	1.4	1.4	0.6	0.6	
B_SW	11		Wind Bent In Wall				

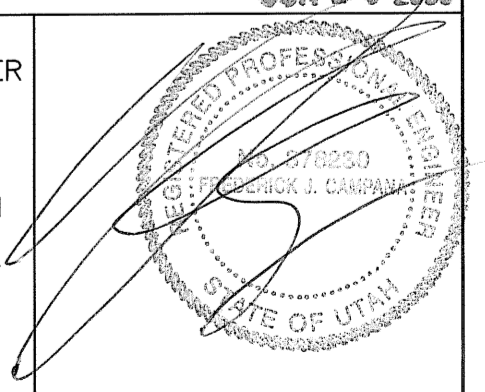
**WIND BENT REACTIONS**

Loc	Wall Line	Col Line	± Reactions			
			Wind(k)	Seismic(k)	Horz	Vert
F_SW	7	I	1.05	1.06	1.36	1.37
F_SW	7	J	1.05	1.06	1.36	1.37
B_SW	11	J	1.11	1.04	1.34	1.25
B_SW	11	I	1.11	1.04	1.34	1.25

**FOR PERMIT**

JUN 10 2008

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ISSUE	REV.	DESCRIPTION	DATE	DRN.	DES.
A		CONFIRMATION	06.20.08	FFF	DD
0		CONSTRUCTION	06.04.08	DM	MT

**RIGID BUILDING SYSTEMS**  
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 Houston, Tx. 77073  
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DESCRIPTION	FRAME REACTIONS
CUSTOMER	ELEGANTE ENTERPRISES, INC.
END USER	PAINTERS SERVICE CENTER
END USE	OFFICE/SERVICE BAYS
LOCATION	1600 HILTON DR. ST. GEORGE, UT 84770
DRN. BY: DM	DATE: 06/04/08
CHK. BY: MT	DATE: 06/04/08
JOB NO.: 28268-C	SCALE: N.T.S.
	DWG. NO.: F3 OF 3
	ISSUE: 0