



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)						Anc. Bolt No D(in)	Base Plate (in)			Grout (in)	
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin		Wid	Len	Thk		
6	A	13	10.3	17.7	8	-9.4	-10.4	4	0.750	8.000	13.50	0.500	0.0
6	H	9	4.6	-3.0	12	-3.8	1.9	4	0.750	8.000	13.00	0.500	0.0
6	C	7	0.0	-28.6	7	0.0	-28.6	4	0.750	8.000	8.500	0.500	0.0
6	E	6	0.0	-8.2	6	0.0	-8.2	4	0.750	8.000	8.500	0.500	0.0
6	F	6	0.0	-9.5	6	0.0	-9.5	4	0.750	8.000	8.500	0.500	0.0

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		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin		Wid	Len	Thk		
8 *	A	15	8.4	21.4	8	-6.0	1.0	4	0.750	8.000	13.00	0.500	0.0
8 *	H	9	6.4	-21.9	14	-7.2	21.0	4	0.750	8.000	11.00	0.500	0.0

8 * Frame lines: 8 10

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Frm Line	Col Line	Column Reactions (k)						Anc. Bolt No D(in)	Base Plate (in)			Grout (in)	
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin		Wid	Len	Thk		
12	A	13	7.9	16.0	8	-6.6	-7.8	4	0.750	8.000	11.50	0.500	0.0
12	H	9	2.5	-1.3	12	-2.2	1.4	4	0.750	8.000	11.50	0.500	0.0
12	B	7	0.0	-13.1	7	0.0	-13.1	4	0.750	8.000	8.500	0.500	0.0
12	D	6	0.0	-7.0	6	0.0	-7.0	4	0.750	8.000	8.500	0.500	0.0
12	G	6	0.0	-2.3	6	0.0	-2.3	4	0.750	8.000	8.500	0.500	0.0

ENDWALL COLUMN: REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)						Out-Of-Plane	
		Dead Vert	Coll Vert	Live Vert	Wind-Left Horiz	Wind-Right Horiz	Wind P	Wind S	
6	C						-2.7	3.1	
6	E						-2.8	3.2	
6	F						-2.9	3.3	
12	G	1.4	0.8	5.0	0.0	-5.0	0.0	-3.6	4.0
12	D	1.2	0.7	4.6	0.0	-4.6	0.0	-3.0	3.3
12	B	0.7	0.3	2.2	0.0	-2.2	0.0	-2.0	2.3

SOLDIER COLUMN: REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)						Out-Of-Plane	
		Dead Vert	Coll Vert	Live Vert	Wind P Horiz	Wind S Horiz	LnWind	LnWind	
7	H	(USE REACTION ON LEFT E.W. COLUMN OF BLDG. - 'C')							

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)						Anc. Bolt No D(in)	Base Plate (in)			Grout (in)	
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin		Wid	Len	Thk		
6	C	18	3.0	-5.4	19	-2.7	-3.9	4	0.750	8.000	8.500	0.500	0.0
6	E	18	3.2	-4.7	19	-2.8	-3.2	4	0.750	8.000	8.500	0.500	0.0
6	F	18	3.3	-5.2	19	-2.9	-3.9	4	0.750	8.000	8.500	0.500	0.0
12	D	18	4.0	-5.0	19	-3.6	-4.3	4	0.750	8.000	8.500	0.500	0.0
12	G	18	3.3	-4.5	19	-3.0	-3.8	4	0.750	8.000	8.500	0.500	0.0
12	B	18	2.3	-2.2	19	-2.0	-1.8	4	0.750	8.000	8.500	0.500	0.0

SOLDIER COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k)						Anc. Bolt No D(in)	Base Plate (in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin		Wid	Len	Thk	
7	H	(USE REACTION ON LEFT E.W. COLUMN OF BLDG. - 'C')										

BRACING REACTIONS, PANEL SHEAR

Frm Line	Col Line	± Reactions (k)				Panel Shear (lb/ft)
		Wind Horiz	Wind Vert	Seismic Horiz	Seismic Vert	
L_EW 6	Rigid Frame At Endwall					
F_SW H	Wind Bent In Wall	10.8	10.5	12.8	23.5	
R_EW 12	Rigid Frame At Endwall					
B_SW A						

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind_L1		Wind_R1	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
6	A	0.07	4.44	0.01	0.28	0.18	9.28	-0.04	1.87	-8.11	-14.28	10.22	13.00
6	H	-0.07	4.73	-0.01	0.29	-0.18	11.45	0.04	5.84	-3.71	-3.17	3.15	-7.13
6	C	0.00	9.25	0.00	1.08	0.00	24.80	0.00	4.70	0.00	0.51	0.00	-34.17
6	E	0.00	8.64	0.00	1.60	0.00	19.72	0.00	4.13	0.00	-13.38	0.00	-9.08
6	F	0.00	9.93	0.00	1.07	0.00	24.48	0.00	4.00	0.00	-15.43	0.00	-5.37

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data.

Width (ft)	: 60
Length (ft)	: 60
Eave Height (ft)	: 26 / 28.5
Roof Slope (rise/12)	: 0.5/12
Design Code	: IBC 06
Enclosure	: Closed
Dead Load (psf)	: 2
Collateral Load (psf)	: 3
Wind Speed (mph)	: 90 mph
Wind Importance Factor	: 1.00
Wind Exposure	: C
Live Load (psf)	: 20
Frame Live Load (psf)	: 12
Ground Snow Load (psf)	: 21.000
Roof Snow Load (psf)	: 20
Snow Exposure	: 1.000
Snow Importance Factor	: 1.000
Thermal Factor	: 1.000
Seismic Importance Factor	: 1.00
Spectral Response Accel.	: Ss=0.511
Spectral Response Coeff.	: Sds=0.474
Seismic Coeff. (Fa*Ss)	: 0.710
Seismic Design Category	: D

5. Loading conditions are:

- DL+CL+LL
- DL+CL+SL
- DL+CL+0.75LL+0.75WL2
- DL+CL+0.75LL+0.75WR2
- DL+CL+0.75LL+0.75LnWind2
- 0.60DL+WL1
- 0.60DL+WR1
- 0.60DL+WL2
- 0.60DL+WR2
- 0.60DL+LnWind1
- 0.60DL+LnWind2
- DL+CL+WL1
- DL+CL+WR1
- 1.07DL+1.07CL+0.70SeisL
- 1.07DL+1.07CL+0.70SeisR
- 1.05DL+1.05CL+0.75LL+0.52LnSeis
- DL+CL+SL/2+F3PAT_SL 4
- 0.60DL+WR1+WS
- 0.60DL+WP+LnWind1
- DL+CL+SL+WP
- 0.60DL+WS

WIND BENT REACTIONS

Loc	Wall Line	Col Line	± Reactions (k)			
			Wind Horiz	Wind Vert	Seismic Horiz	Seismic Vert
F_SW	H	8	4.78	12.79	12.41	32.90
F_SW	H	10	4.78	12.79	12.41	32.90

FOR PERMIT

JUN 10 2008

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT RIGID BUILDINGS ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY RIGID IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN RIGID ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

RIGID BUILDING SYSTEMS

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ISSUE	REV.	DESCRIPTION	DATE	DRN. DES.
A		CONFIRMATION	05.20.08	FFF
0		CONSTRUCTION	06.04.08	LK MT

DESCRIPTION	FRAME REACTION
CUSTOMER	ELEGANTE ENTERPRISES, INC.
END USER	PAINTERS SERVICE CENTER
END USE	OFFICE/SERVICE BAYS
LOCATION	1600 HILTON DR. ST. GEORGE, UT 84770
DRN. BY:	LK
DATE:	06/04/08
CHK. BY:	MT
DATE:	06/04/08
JOB NO.:	28268-A
SCALE:	N.T.S.
DWG. NO.:	F3 OF 3
ISSUE:	0